

# Concrete with Reused Concrete Sand

Mohankumar Namdeorao Bajad



**Abstract:** This paper examines the impacts of substitution of reused concrete sand (RCS) with sands, on the new and hardened physiognomies of concrete. the property of RCS blended concrete was examined and likened with ordinary concrete of 40 MPa compression strength. the physiognomies of RCS concrete vary from ordinary concrete arranged with characteristic sand, as an outcome of the quality of connected mortar, old cement glue, and more fines. the outcomes demonstrate that the RCS concrete demonstrations tantamount workability in contrast with ordinary concrete. the mechanical physiognomies (compressive, flexure, split tensile and elastic modulus) of concrete developed with RCS was lower in compression to ordinary concrete however worthy up to 60percentage RCS in the blend. The drying shrinkage strain of 100percentage RCC mixed concrete at twenty-eight days was watched twice in compression to controlled concrete and it demonstrated more abrasion value in that comparison and furthermore concrete developed with 100 percent RCS indicated 41percentage and 11.3percentage lower in sorption value at ahead of schedule and later age organize individually in that examination.

**keywords:** Reused aggregate; reused concrete sand sorptivity.

## I. INTRODUCTION

The development in the monetary advancement of the framework, for sample, Streets, Bridges, Flyovers, Subways, and so forth condition has come to at a caution organize in numerous urban areas, which has led to an expanding measure of development and destruction squander (D&DS). In India, D&DS has created around 14.5 million tons yearly detailed by Esa, M. R., Halog, A., & Rigamonti, L. (2016) [1]. At this stage, there is interest for continued advancements through the decline of D&DS sum. Heaps of D&DS along the streets and in empty terrains seen in numerous urban communities in India are the amazing wellspring of aggregates and sand for new concrete. Numerous specialists have utilized reused aggregate in concrete and the massive mainstream of the work has concentrated on supplanting the coarse aggregate by reused concrete aggregate (RCA) and furthermore intrigued by the examination concerning the physiognomies of concrete developed with RCA. In any case, Khatib (2005) [2] concentrated that concrete developed with the reused concrete sand (RCS) and found that the is more water absorption increment chance in the last aftereffects of concrete. A few scientists have detailed that the fine grating of RCA, may impact contrarily on the presentation of concrete. Mesbah and Buyle-Bodin (1999) [3] presumed that extraordinary porosity and more water ingestion of RCS demonstrates movement instrument of water and is the fundamental driver of drying shrinkage. Poon and Kou (2010)

[4] announced that more water devoured by RCS than characteristic sands and furthermore demonstrate that mortar comprising a higher level of RCS substance have lower mechanical strength than a mortar with regular fines aggregates. Pelufo et al. (2009) [5] presumed that the quantity of water to bond (w/c) assumes a significant job in concrete and outcomes demonstrated increment in compression strength as RCS substance expanded when the normal aggregate was utilized in concrete. Lee, Seung-Tae, et al. (2008) [6] indicated valuable impacts of MgSO4 obstruction attack on mortar samples made with half substitution of RCS and at 100percentage substitution demonstrated negative MgSO4 attack on mortar samples. This paper examined the discoveries of definite test study planned for researching the physiognomies of concrete with zero percentage, twenty percentage, fourth percentage, sixty percentage, eighty percentage, hundred percentage RCS (therein alluded as R0, R20, R40, R60, R80, R100 separately) and contrasted the equivalent and ordinary concrete (R0) intended for trademark compression strength of 40 MPa. Discoveries from this examination as compression strength, flexural and split the resistance of a material to breaking under tension., elastic modulus, drying shrinkage, sorptivity and abrasion of the area of RCS blended concrete and ordinary concrete blends have been talked about.

## II. TEST

### A. Material

The fixings were water, sand, aggregate, fine reused aggregate (reused concrete sand) and conventional Portland cement 43 evaluation was utilized in both controlled concrete and RCS blended concrete. The physical physiognomies have appeared in Table 1. The sand was normal sand gathered from neighbourhood assets, with finesses modulus of 2.19 and a relative density of 2.28 complying with IS 383 specifications [7]. 20 mm and 10 mm dimension rock squashed stone with the relative density of 2.61 and 2.60 and water retention were 0.32 and 0.36 percentage separately were utilized for making the concrete blend.

**Table-I: Test outcomes of ORDINARY PORTLAND CEMENT 43 grade cement**

Description	Normal Consistency (percentage)	Initial Setting Time (Min.)	Final Setting Time (Min.)	Avg. Compression Strength at twenty-eight days (MPa)
Ordinary Portland Cement 43 Grade	27.5percentage	91	212	58.4

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## Recycled concrete sand

In this assessment, the point of this exploration was the creation of reused concrete sand to making concrete for concrete streets. The samples of reused concrete sand (RCS) utilized were gathered from development and destruction squander (D&DS) squashing plant at Burari in North Delhi between Aug. to Sept. 2016.

This D&DS pulverizing plant had an arrangement of waste administration by isolating old concrete, artistic squares, tiles and earth and so forth from D&DS, at that point squashed old concrete, independently in effect smasher to produce the diverse dimension of aggregates and sand. The physical physiognomies of normal sand (NS) and reused concrete sand have appeared in Table 2.

**Table-II: Physical physiognomies of NS and RCS**

Physiognomies	NS	RCS
Relative density	2.29	1.96
Bulk Unit weight of loose state (kg/m <sup>3</sup> )	1.438	1.22
Bulk Unit weight of compacted state (kg/m <sup>3</sup> )	1.658	1.48
Water absorption (percentage)	4.15	12.50

## B. Mix quantity's

The blend extents for controlled concrete intended for 40 MPa trademark compression strength and RCC mixed concrete are displayed in Table 3. In this investigation, every single normal aggregate and RCS were in the same conditions. i.e., as got in air-dried and did not prewet or pre-absorb grouping or blending since Poon et al. (2004) [8] and de Oliveira and Vazquez (1996) [9] expressed that concrete, arranged with soaked surface and air-dried surfaced reused concrete aggregate demonstrated a distinction in

**Table-III: Mix quantity of PQC (kg/m<sup>3</sup>)**

Mix type	W/C	Cement	Sand	RCS	20mm	10mm	Water	Admixture	Slump (mm)
CC	0.39	423	551.2	-	536.3	655.4	165	0.7614	66
RCS-20			440.9	110.3					54
RCS-40			330.7	220.5					42
RCS-60			220.5	330.7					36
RCS-80			110.3	440.9					28
RCS-100			-	551.2					15

## A. Unit weight and absorption of concrete

The unit weight and voids of concrete were resolved at 28 years old days as per ASTM C642[11]. The round and hollow samples of dimensions 100 mm diam and 200 mm tallness were casted from controlled concrete and RCS blended concrete mixture, following 24 hours of throwing, demould sample shape, stamped and kept in water for relieving at coldness (27±2°C) till the ideal period of testing.

## B. Compression strength

For the compression strength testing, the cube samples of dimension 150×150×150 mm were thrown from controlled concrete and RCS blended concrete mixture and compacted on vibrating table, next twenty-four hours of throwing samples form were demoulding, set apart for distinguishing proof and kept in water tank for relieving at coldness (27±2°C) till 7, 28- and 56-days' period of testing.

## C. Bend strength

The inflexible asphalt flop in twisting, because of this asphalt unit weight configuration dependent on bend strength (modulus of rupture) of concrete, hence bend strength was

compression strength. A steady w/c quantity of 0.39 was utilized for controlled concrete and RCC mixed concrete blends which partitioned into five gatherings that were 20percentage, 40percentage, 60percentage, 80percentage, 100percentage of RCS.

## C. Preparation of samples

A turning drum mixture was utilized to get ready concrete in research facility, first the sand and coarse aggregate were put in mixture and blended for 60 sec then ½ of all-out amount of second era polycarboxylic ether polymers (PCE) based super-plasticizer was blended for surmised 60 sec, because of this, aggregate particles assimilated some portion of water and wetted, after that concrete and remained PEC based super-plasticizer was added to blend and blending proceeded for 120 to 180 sec till to get slump between 20 to 40 mm. From each gathering blend, the samples for mechanical and sturdiness test were cast and compacted. The sample molds were demould following 24 hours of throwing and set apart for recognizable proof at that point kept in water tank for relieving at coldness (27±2°C) till the duration of testing.

## III. TESTING OF SAMPLES

In this investigation, workability of 0percentage, 20percentage, 40percentage, 60percentage, 80percentage, 100percentage of RCC mixed concrete was resolved in term of the slump test as indicated by ASTM C143[10] and slump value was estimated following blending of concrete fixings and seeping on the surface of concrete blend was made a decision by outwardly.

resolved at the age of seven, twenty eight and fifty six days as indicated by ASTM C78[12] by beam sample of dimension 100×100×500 mm were cast with controlled concrete and RCS blended concrete mixture and compressed on vibrating table, next twenty-four hours of throwing samples shape were demoulded, stamped and kept in water for restoring at coldness (27±2°C) till seven, twenty-eight and fifty-six days period of testing.

## D. Split the resistance of a material to breaking under tension. and elastic modulus

The split the resistance of a material to breaking under tension. of concrete was resolute at the age of seven, twenty-eight and fifty-six days as per ASTM C496[13] and elastic modulus was decided at 28 days' time of relieving as indicated by ASTM C469[14]. The round and hollow samples of dimension 150 mm diam and 300 mm stature were cast with controlled concrete and RCC mixed concrete then demould, stamped and stored in water for relieving at coldness (27±2°C) till the duration of testing.

**E. Drying Shrinkage**

Because of dampness variety in new and hardened state concrete, the shrinkage tractable anxieties were created in concrete and show up as breaks in concrete, thusly it wound up important to direct drying shrinkage test. The beam samples of dimension 75×75×275 mm were cast for the wanted time of seven and twenty-eight days of testing according to ASTM C157[15]. The hardened steel studs in the focal point of part of the arrangement were given to gauge the length, utilizing the length compactor gadget. next twenty four hours samples were demoulded, stamped and kept in a water tank at coldness (27±2°C) till the duration of testing. After taking the underlying perusing, samples were dried at 50±1°C coldness with 17percentage relative mugginess. The samples were unprotected to cycles of drying and cooling for estimating length changes until consistent length accomplished. The underlying drying shrinkage was determined as the underlying distinction between perusing of reference bar and samples were recorded following demould and the length change at wanted age was determined by the contrast between compactor perusing (CRD) and introductory CRD perusing. The strain was determined as the length change isolated by the check length of 254 mm, normal drying shrinkage (normal of three) is talked about with outcomes

**F. Sorptivity**

The barrel-shaped samples of dimension 100 mm diam × 200mm tallness were cast with ordinary concrete and RCS blended concrete mixture. Demoulded the samples, set apart for recognizable proof and kept in water at coldness (27±2°C) until the duration of testing. A circle of dimension 100 mm diam × 50mm tallness cuts from threw chamber for the assurance of pace of water retention at 28 years old days as indicated by ASTM C1585[16].

**G. Abrasion of concrete**

On account of the higher traffic volume of overwhelming hub stacks, the concrete asphalts were unprotected to increasingly surface scraped spot; hence, abrasion opposition had incredible significance for concrete streets. The abrasion opposition test can be performed on concrete streets in the arena or on section test in the lab. 800×800×100 mm dimension of section samples was thrown with ordinary concrete and RCC mixed concrete for the wanted time of 28 days of testing as per ASTM C779[17]. The flat abrasion machine pivots in a round way at a speed of 12 cycles for every moment, during the revolution, silicon power falls at the pace of 4 to 6 gm/min from the cup attached to the top of the shaft, which rubs the outside of the piece. The underlying perusing was taken after 5 min with the assistance of micrometer and after pivot for 30 minutes, second readings were taken then again rotation for 30 minutes the last perusing were taken. The distinction between normal introductory and normal last perusing gives a complete abrasion in millimetres.

**IV. OUTCOMES AND DISCUSSION**

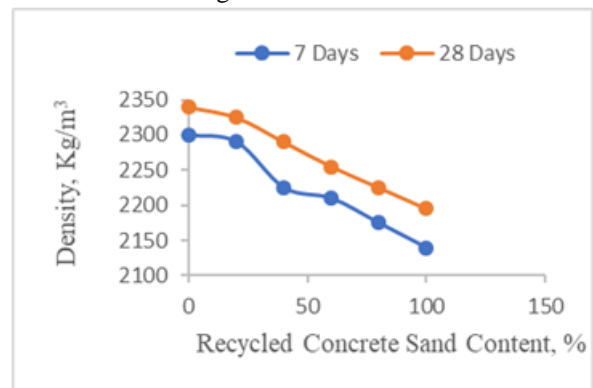
**A. Workability**

Table 3 introduces the aftereffects of slump worth and measure of water blended in the blend of concrete. It was seen that the RCC mixed concrete required more water for blending than ordinary concrete, on the grounds that reused concrete sand has exceptionally high-water retention, which

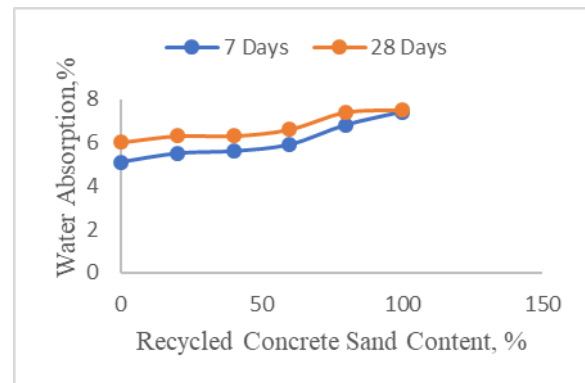
was around 3-4 times more noteworthy than common sand due to profoundly rakish and all the more unpleasant surface as contrasted and characteristic sand. Anyhow, the workability of crisp concrete appeared to be diminished on expanding the level of RCS in the concrete blend. This is expected to RCS had a lower relative density and better particles than characteristic sand. So, RCS created more prominent volume prompting a decrease in the measure of blending water, bringing about a decline in workability of concrete.

**B. Unit weight and absorption of concrete**

Figure 1 introduces the unit weight in kg/m<sup>3</sup> for all concrete blends at the age of seven and twenty-eight days relieving. It was seen that valuation of unit weight extended from 2135 to 2340 kg/m<sup>3</sup> for all concrete blends, in this way the worth acquired at 28 days, unit weight of 2340 kg/m<sup>3</sup> for R0 concrete and R100 concrete has the unit weight of 2190 kg/m<sup>3</sup>. A diminishing in unit weight can be seen as the expansion in substitution of normal sand by RCS. Therewith, it was likewise seen that on expanding the level of substitution of RCS in the concrete blend that lead to more water retention and misfortune in the valuation of unit weight. The 6.04percentage and 8.02percentage assimilation worth acquired at twenty-eight days restoring for R0 concrete and the R100 concrete individually. The 100percentage RCC mixed concrete demonstrated 24.7percentage more water absorption than R0 concrete. In this manner, the outcomes show that the w/c quantity is the fundamental factor that influences the unit weight and water retention of concrete.



**Fig. 1(a). Variation in unit weight at seven and twenty-eight days of curing**



**Fig. 1(b). Variation in absorption at seven and twenty-eight days of curing**

### C. Compression strength

The compression strength of concrete samples was tried at seven, twenty-eight and fifty-six days. Figure 2 clarifies the advancement of compression strength in samples with various level of substitution of RCS in the concrete blend. at the point when RCS substance was expanded from 0 to 100percentage in concrete blend, a straight drop in strength was seen at seven, twenty eight and fifty six days' time of relieving, in light of the fact that RCS contains bond past particles and more artfulness in contrast with natural sand which create porosity that lessen compression strength, which impacts mechanical physiognomies of concrete developed with RCS, despite the fact that the contrasts between 20percentage, 40percentage and 60 percentage RCC mixed concrete were little since Poon et al. (2004)[8] has been proposed a slight protection from misfortune in concrete developed with reused sand and Barra de Oliveira and Vazquez (1996)[9] expressed that the strength of concrete may be influenced with immersed reused aggregate, in light of the fact that at higher immersion levels, the bond between concrete glue and reused aggregate become flimsier. In this manner, it was seen from outcomes that at the 80percentage or greater supplanting of RCS with regular sand in the concrete blend may have led to flimsier execution of that concrete.

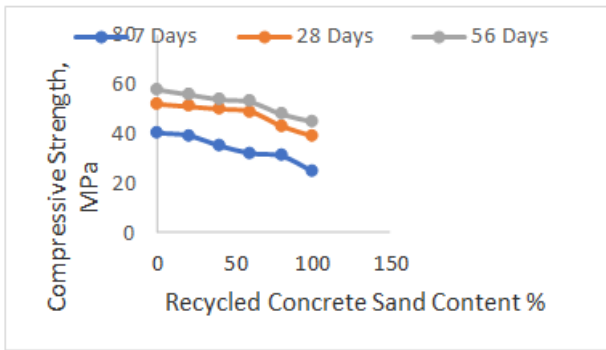


Fig.2. Variation in compression strength at 7, 28- and 56-days age curing

### D. Split tensile and bend strength

The split tensile and bend strength of the RCC mixed concrete at 7, 28- and 56-days' time of relieving are exhibited in Figure 3, which demonstrates a reasonable loss of this property with the expanding of RCS supplanting with normal sand. As appeared in Figure 3 the flexural and split the resistance of a material to breaking under tension. at age of seven, twenty-eight and fifty-six days diminished with expanding the level of substitution of RCS on account of the permeable surface of following mortar and old bond glue on RCS. The IS 456[18] recommended a connection among flexural and compression strength of concrete is given by equation (1).

$$f_b = 0.7\sqrt{f_{ck}} \dots (1)$$

From Figure 3, it is understood that the valuation of the split tensile and bend strength of RCC mixed concrete at twenty-eight days old and fifty-six days more than strength determined from compression strength by equation (1)

### E. Elastic modulus

The elastic modulus is gotten at 28 years old long stretches of restoring of round and hollow concrete samples. The drop

in the valuation of modulus of the elasticity of RCC mixed concrete in contrast with R0 concrete when the RCS rate expanded in the concrete blend. Zaharias et al. (2004) [19] has been shown that the decrease in the valuation of the elastic modulus up to 45percentage, when 100percentage reused aggregate was utilized in examination with ordinary concrete. Figure 4 demonstrates the valuation of the elastic modulus with various RCS substitution, it is seen that the elastic modulus diminishes in a rough straight connection and furthermore demonstrates the misfortune on the valuation of the elastic modulus was 19.3percentage at 100 percentage supplanting of common sand with RCS, which might be owing to security among aggregate and glue, concreteness of mortar and their porosity that closed by Neville (1997)[20] and Evangelista and Brito (2007)[21]. In this manner, it could be reasoned that lower concreteness and bond just as the porosity of reused concrete sand were additionally essentially impacted to decrease the elastic modulus concrete. It was likewise observed from outcomes that the elastic modulus lessens altogether in concrete developed with RCS than ordinary concrete. At 28 years old long stretches of relieving the 60percentage supplanting of natural sand with RCS, the concrete delivered 31.7 GPa elastic modulus which is minimal more or equivalent to value determined from compression strength of ordinary concrete by equation (2) for sample 31.62 GPa at twenty-eight days according to IS 456:2002 [18].

$$E_c = 5000\sqrt{f_{ck}} \dots (2)$$

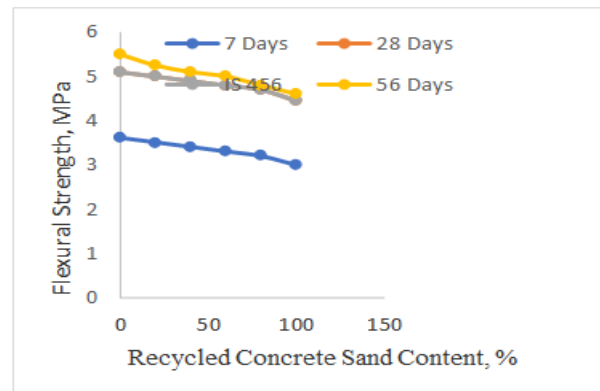


Fig. 3(a). Variation in bend strength at 7,28- and 56-days age of curing

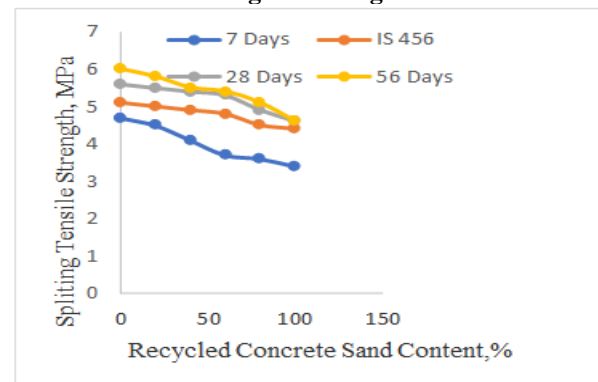
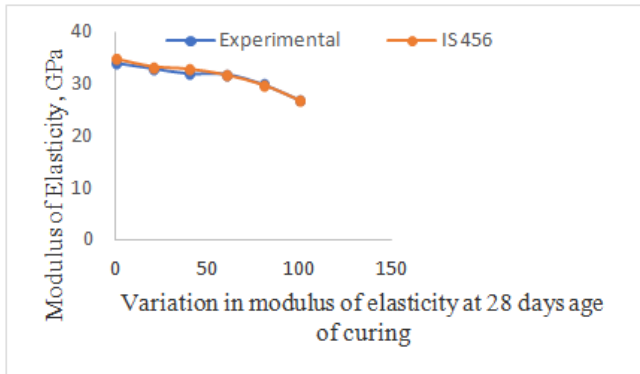


Fig. 3(b). Variation in split the resistance of a material to breaking under tension. at 7,28- and 56-days age of curing

**F. Durability Physiognomies**

**Sorptivity**

The sorptivity assumes a conspicuous job in the inflexible asphalt, which assessed by the pace of the water absorption test. The normal consequences of sorptivity are introduced in Table 4. The concrete blend with various level of RCS displays low sorption in starting and later age arranges in compression to ordinary concrete and outcomes demonstrated that at 100percentage RCS substitution in concrete blend indicated 41percentage and 11.3percentage lower in sorption value at ahead of schedule and later age organize separately in that examination. since the high volume of fine particles in RCS than natural sand that lessens the availability of pores in concrete.



**Fig. 4. Variation in elastic modulus at twenty-eight days age of curing**

**Table-IV: Effect of RCS replacement in concrete mix on sorption coefficient**

Percentage of RCS in mix	Early age sorption coefficient (mm/min <sup>1/2</sup> )	Later age sorption coefficient (mm/min <sup>1/2</sup> )
0	0.212	0.0048
20	0.197	0.0044
40	0.161	0.0035
60	0.173	0.0031
80	0.145	0.0027
100	0.125	0.0024

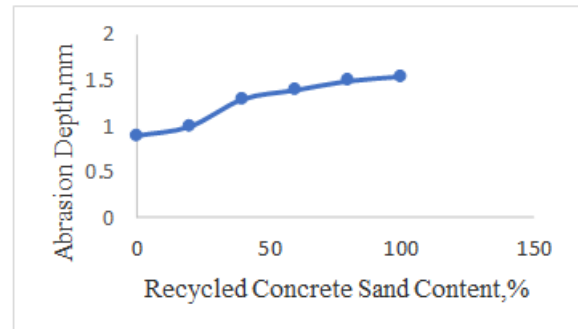
**Abrasion of concrete**

The abrasion of concrete got at 28 years old long periods of relieving. Figure 5 demonstrates the valuation of the abrasion with various level of RCS substitution in a concrete blend. It was seen that the expansion in the abrasion value an incentive with increment in the level of RCS and furthermore it tends to be seen from Figure 5 that abrasion value is higher by 67percentage at 100percentage RCS in contrast with 0percentage RCS in the concrete blend. This might be due to of ingestion of more water by permeable mortar follow on RCS particles that make week bond in mortar and increment in abrasion value.

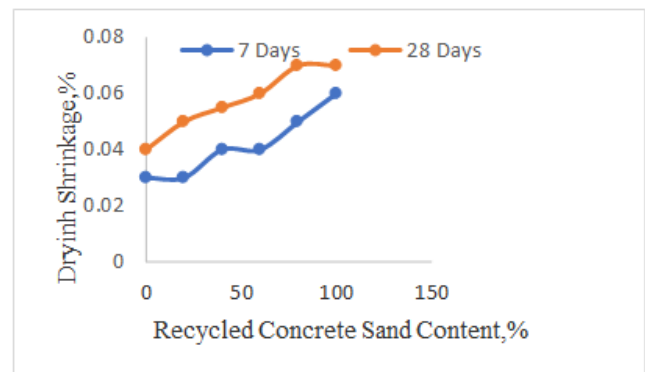
**Drying Shrinkage**

The drying shrinkage of samples of various RCS concrete is estimated at 7 years old and 28 days of restoring of samples are exhibited in Figure 6. It very well may be seen from Figure 6 that the drying shrinkage strain of R100 concrete at seven and twenty-eight days of relieving was 0.060percentage and 0.071percentage individually, consequently the R100 concrete at twenty-eight days was 15.5percentage higher in

incentive than 7 days restored R100 concrete. This might be due to the high volume of reused concrete fines in RCS, which produce high porosity in concrete because of holding more water in new concrete that was closed by Kou et al. (2007) [22]. For the RCS blended concrete, the expansion in drying shrinkage maybe because of old bond substance and mortar sticking to RCS particles.



**Fig.5. Variation in abrasion of concrete at twenty-eight days age of curing**



**Fig.6. Variation in drying shrinkage of concrete**

**V. CONCLUSIONS**

In light of the test examination of the impact of RCS on concrete, the accompanying ends can be made:

1. The workability of concrete abatement as the level of reused concrete sand expands, The RCS supplanting up to 60percentage with common sand in the concrete blend has fulfilled the serviceable concrete with no draining and isolation.
2. The compression strength does not appear to be influenced by expanding reused concrete sand up to 60percentage at twenty-eight days period of restoring.
3. Both flexural and split the resistance of a material to breaking under tension. is decreased with expanding RCS rate in the concrete blend: in any case, the worth got for the two physiognomies are as yet worthy up to 60 percentage RCS in the concrete blend at twenty-eight days period of restoring.
4. The elastic modulus dropped when RCS substitution rate increment in the concrete blend, however, does not is by all accounts influenced up to 60percentage RCS substitution at 28 days' time of restoring.
5. The abrasion opposition is by all accounts increment with the substitution of RCS in the concrete blend.

6. Drying shrinkage of RCC mixed concrete at twenty-eight days was about twice in compression to ordinary concrete
7. The RCC mixed concrete was observed to be less porous, which shows lower water absorption and the low valuation of sorptivity coefficient value in compression to ordinary concrete.

#### NOMENCLATURE

fb = bend strength of concrete at a given age, MPa  
fck = compression strength of concrete at a given age, MPa  
Ec = elastic modulus of concrete at a given age, GPA

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